

# ESTABLISHMENT OF FUNDAMENTAL CHARACTERISTICS OF SOME UNSATURATED SRI LANKAN RESIDUAL SOILS

Nanthini Vasanthan

(138829P)

Supervised by Prof. S.A.S. Kulathilaka

M.Eng. in Foundation Engineering and Earth Retaining Systems

Department of Civil Engineering

University of Moratuwa

Sri Lanka

(2013/2014 BATCH)

# ESTABLISHMENT OF FUNDAMENTAL CHARACTERISTICS OF SOME UNSATURATED SRI LANKAN RESIDUAL SOILS

Nanthini Vasanthan

(138829P)

Degree of Master of Engineering

Department of Civil Engineering

University of Moratuwa Sri Lanka

November 2016

# ESTABLISHMENT OF FUNDAMENTAL CHARACTERISTICS OF SOME UNSATURATED SRI LANKAN RESIDUAL SOILS

Nanthini Vasanthan

(138829P)

Thesis submitted in partial fulfillment of the requirements for the degree of Master of Engineering in Foundation Engineering and Earth

Retaining Systems

Department of Civil Engineering

University of Moratuwa Sri Lanka

November 2016

#### **DECLARATION**

I declare that this is my own work and this thesis does not incorporate without acknowledgement any material previously submitted for a Degree or Diploma in any other University or Institute of higher learning to the best of our knowledge and belief it does not contain any material previously published or written by another person except where the acknowledgement is made in the text.

Also, I hereby grant to University of Moratuwa the non-exclusive right to reproduce and distribute my thesis, in whole or in part in print electronic or other medium. I retain the right to use this content in whole or part in future works (such as articles or books)

Signature of the candidate:	Date:
Nanthini Vasanthan	
The above candidate has carried out research for the supervision.	e Master Dissertation under my
Signature of the supervisor:	Date:

Professor S.A.S. Kulathilaka, B.Sc. Eng. Hons (Moratuwa), Ph.D. (Monash), C.Eng., MIE(SL), Department of Civil Engineering, University of Moratuwa, Sri Lanka.

#### **ABSTRACT**

Slope failure in tropical climates frequently occurs due to excessive rainfall. Heavy infiltration causes destruction of matric suctions, development of perched water table conditions and rise of ground water table. Severe erosion and surface destruction will also be caused by the heavy prolonged rainfall. In order to understand the threshold values of rainfall leading to instability it is necessary to model this process with a reasonable accuracy.

Sri Lankan residual soil formations are formed by weathering of the metamorphic parent rock and have inherited significant abrupt variations in engineering characteristics. Basic characteristics of these soil formations such as soil water characteristic curves (SWCCs), variation of permeability with water content and unsaturated shear strength parameters are essential parameters in these analyses. These characteristics have not been established for typical residual soils forming slopes in Sri Lanka.

This thesis highlights the need for detailed experimental studies and presents comprehensive studies that have been conducted at the University of Moratuwa and National Building Research Organization (NBRO) laboratories to establish the fundamental characteristics of unsaturated Sri Lankan residual soils. Undisturbed samples of soil obtained from the failed slope at Welipenna in the Southern Expressway were used in this study.

Direct shear tests were done by modifying the conventional apparatus by incorporating a miniature tensiometer which allows for the simple and direct measurement of soil matric suction during shearing. Soil water characteristic curves (SWCCs) were also established using these apparatus. Alternatively, pressure plate apparatus was also used for this purpose. In addition to that, soil water characteristic curves (SWCCs) were developed from gradation curve also.

Permeability of an unsaturated soil varies considerably with the level of saturation and will make a very significance influence on the infiltration process. Permeability function which defines the variation of permeability with matric suction was investigated on undisturbed samples. The method is based on continuously drying and wetting the soil sample while continuously monitoring the suction gradient and the change in soil mass. The thesis highlights the importance of these studies and presents the procedures that are being used.

### **DEDICATION**

This thesis is dedicated to my parents Late Mr.N.Vethasalam and Mrs.V.Sakunthaladevi. They have encouraged me all the time "study, study, study......, then only you will achieve your target". As they said I have obtained my BSc. Eng. (Civil) in March 2004 and I started my master carrier in May 2013. I have lost my father in March 2015 during my masters. Appa! your dream came true and I know that you are somewhere around here watching our achievements.

#### ACKNOWLEDGMENTS

I would first and foremost like to thank my Lord, who has made this dream and all other realities possible.

First and foremost, I would like to express my deepest gratitude to Prof. Athula Kulathilaka, Professor at Civil Engineering Department of University of Moratuwa, for his dedication and keen interest to make success and as well as for setting me the platform to proceed my career in Geotechnical Engineering. I am highly indebted to Prof. Apiniti Jotisankasa, the head of the Geotechnical Engineering section of Kasetsart University, Thailand not only for supplying instruments and giving training on tensiometer at Universities of Kasetsart and Moratuwa but also clarifying the doubts, problems regarding the instruments and research throughout my research carrier. Without their guidance and persistent help this drive would not have been possible.

My sincere gratitude should go to Mr. Narin Hunsachainan, Assistant Lecturer of Civil Engineering Department of Kasetsart University and Mr. Ajintit, Masters Student, Kasetsart University for giving in hand training on installing and handling the tensiometer.

I should really pay my sincere gratitude to Eng. (Dr) Asiri Karunawardena, Director General of National Building Research Organization (NBRO), for his guidance and continuous support throughout the masters. It is my obligation to acknowledge the commitment and the courage given by Mr. K.N Bandara, Director of Geotechnical Engineering Division (GED) and all the other staff of Geotechnical Engineering Division of NBRO. In this regard I should especially acknowledge the assistance given by Mr. Lilanka Kankanamge, Engineer, Ms. Nirmanthi Idirimanne, Engineer and Ms. Chamila Jayasundara, Geologist, NBRO.

The assistance received from Mr.K.R.Pitipanaarachchi, Technical Officer, Mr. D.G.S Vithanage, Technical Officer, Mr. M.A.Ajith Piyasiri, Lab Assistant and Ms. Pradeepa Pieris, M.Eng. Course Assistant in the Soil Mechanics Laboratory, University of Moratuwa and similarly, Mr. Chandima Piumkith, Management Assistant-Technical, Mr.M.Abeysinghe, Management Assistant-Technical during the laboratory testing programme, and all other laboratory staff who have supported conducting all kind of laboratory tests in Geotechnical Engineering Laboratory, NBRO is acknowledged. Also I would like to acknowledge the assistance extended by Mr. U.K.Padmaperuma, Technical Officer, Civil Engineering Workshop and Mr.Yomal Dias, Lecturer (on contract), Civil Engineering Department for fabricating experimental setups successfully.

Last but not least, I would like to personally thank my superb husband Mr.S.Vasanthan, Consultant/Geotechnical Engineer, Resource Development Consultants (RDC) who has given tremendous support not only in the situation where it really matters but also research field as he is also in the same area and my lovely son Master.V.Kirthik, student, Royal College, Colombo-07 who has sacrificed lot of things/benefits for me as I am working woman and master student also.

Nanthini Vasanthan, 9<sup>th</sup> November 2016.

## **Table of Contents**

1.0.	INTRODUCTION	1
1.1.	SCOPE OF SOIL MECHANICS	1
1.2.	UNSATURATED SOIL MECHANICS	2
1.3.	ROLE OF CLIMATE	2
1.4.	UNSATURATED SOIL IN NATURE AND PRACTICE	4
1.4.1.	Unsaturated soil in hydrologic cycle	4
1.5.	EFFECT OF NEGATIVE PORE WATER PRESSURE ON UNSATURATED SO	OILS IN
Sri L	ANKA	5
1.6.	UNSATURATED SOIL PROPERTIES	6
1.6.1.	Soil water characteristic curve (SWCC)	7
1.7.	THESIS OBJECTIVES	9
1.8.	THESIS OUTLINE	10
2.0.	LITERATURE REVIEW	12
2.1.	PHASES OF SOIL	12
2.1.1.	Capillarity action	13
2.1.2.	Capillarity in Soils	13
2.2.	WHAT IS MEANT BY MATRIC SUCTION?	14
2.3.	MATRIC SUCTION WHY IT IS IMPORTANT?	15
2.4.	STRESS STATE VARIABLES	16
2.5.	SHEAR STRENGTH THEORY	18
2.5.1.	Saturated shear strength	18
2.5.2.	Unsaturated soil with a planar modified Mohr-Coulomb envelope	19
2.5.3.	Unsaturated soil with a curved matric suction envelope	20
2.6.	MEASUREMENT OF MATRIC SUCTION	23
2.7.	SOIL WATER CHARACTERISTIC CURVE (SWCC)	23
2.7.1.	The capillary saturation zone (boundary effect zone)	25
2.7.2.	The desaturation zone (transition zone)	26
2.7.3.	The residual saturation zone (residual zone)	26
2.7.4.	Air-entry value	26
2.7.5.	Residual water content	27

2.7.6.	The inflection point	27
2.7.7.	The hysteresis of two curves	27
2.8.	MODELLING OF SOIL WATER CHARACTERISTICS CURVE (SWCC)	28
2.9.	ESTABLISHMENT OF SOIL WATER CHARACTERISTIC CURVE (SWCC)	29
2.9.1.	Using KU tensiometer	29
2.9.2.	Using pressure plate apparatus	35
2.9.3.	Determination of permeability function	37
2.9.4.	Estimation of soil water characteristic curve using grain size distribution	1 41
2.9.4.1.	Pore volume and volumetric water content	42
2.9.4.2.	Particle size and pore radius	44
2.9.4.3.	Pore radius and soil water pressure	45
2.10.	ANALYTICAL STUDIES DONE IN SRI LANKA ON INFILTRATION	OF
RAINWA	TER	47
3.0.	GEOLOGY AND BASIC PROPERTIES OF THE SOIL TESTED	51
3.1.	SITE DESCRIPTION AND COLLECTION OF UNDISTURBED SAMPLE	51
3.2.	GENERAL GEOLOGY OF THE PROJECT AREA	51
3.3.	GEOLOGY OF THE SITE	53
3.4.	DETERMINATION OF THE CHARACTERISTICS OF SOILS ENCOUNTERED.	53
3.4.1.	Classification of the soil	54
3.4.2.	Determination of saturation period	56
3.4.3.	Saturated permeability of the soil	56
3.4.4.	Consolidation test	58
4.0. I	DEVELOPMENT OF SWCC WITH PRESSURE PLATE APPARAT	CUS
•		61
4.1.	PRESSURE PLATE APPARATUS	61
4.2.	CONCLUDING COMMENTS	66
5.0. I	DIRECT SHEAR TESTS	68
5.1.	DIRECT SHEAR TESTS WITHOUT SUCTION MEASUREMENTS	68
5.1.1.	Testing procedure	68
5.1.2.	Assumption of same φ' at all levels of saturation	68

5.1.3.	Analysis and results for conventional direct shear test
5.1.3.1.	Sample under fully saturated condition69
5.1.3.2.	Sample with approximately 40% (as it is) saturated condition72
5.1.3.3.	Sample with approximately 65% saturated condition75
5.1.3.4.	Sample with approximately 83% saturated condition78
5.1.4.	Variation of apparent cohesion with saturation and volumetric water
content	(SILTY SAND)81
5.1.5.	Development of angle of shearing resistance due to suction, $\phi^b$ using
pressure	plate apparatus (SILTY SAND)82
5.2.	DIRECT SHEAR TEST WITH MATRIC SUCTION MEASUREMENT USING
TENSION	METER84
5.2.1.	KU T3 Tensiometer84
5.2.1.1.	Tensiometer description and data transferring to computer84
5.2.2.	Water saturation and de-airing process
5.2.3.	Testing procedure
5.2.4.	Assumption of same φ' at all levels of saturation88
5.2.5.	Analysis and results for conventional direct shear test with tensiometer 88
5.2.5.1.	Sample under fully saturated condition89
5.2.5.2.	Sample with approximately 50% saturated condition93
5.2.5.3.	Sample with approximately 65% saturated condition94
5.2.5.4.	Sample with approximately 72% saturated condition99
5.2.5.5.	Sample with approximately 81% saturated condition105
5.2.5.6.	Sample with approximately 92% saturated condition111
5.2.6.	Variation of apparent cohesion with saturation and volumetric water
content	(SANDY SILT)117
5.2.7.	Development of angle of shearing resistance due to suction, $\phi^b$ using
pressure	plate apparatus (SANDY SILT)118
5.2.8.	Development of angle of shearing resistance due to suction, $\phi^b$ using
tensiom	eter apparatus (SANDY SILT)
5.3.	CONCLUDING COMMENTS
6.0. D	DEVELOPMENT OF PERMEABILITY FUNCTION125

6.1.	DRYING METHOD	127
6.1.1.	For soil type 01 - SILTY SAND	128
6.1.2.	For soil type 02 - SANDY SILT	130
<b>6.2.</b>	WETTING METHOD	133
6.2.1.	For soil type 01- SILTY SAND	134
6.2.2.	For soil type 02 - SANDY SILT	136
6.3.	CONCLUDING COMMENTS	141
7.0.	DEVELOPMENT OF SWCC WITH DIFFERENT TECHNIC	QUES143
7.1.	ESTABLISHMENT OF SWCC USING PRESSURE PLATE APPARATU	JS143
7.2.	ESTABLISHMENT OF SWCC USING DIRECT SHEAR TESTS WI	TH MATRIC
SUCTI	ION MEASUREMENT	144
7.3.	ESTABLISHMENT OF SWCC USING PERMEABILITY FUNCTION .	146
<b>7.4.</b>	DERIVATION OF SWCC USING THE GRADATION CURVES	148
7.5.	COMPARISON OF DIFFERENT TECHNIQUES	153
8.0.	SUMMARY AND CONCLUSIONS	155
8.1.	FUTURE OUTLOOK	157
9.0.	REFERENCES	159

## List of figures

Figure 2.1: Four phases of unsaturated soils
Figure 2.2: Idealization of soil pores as cylindrical capillaries
Figure 2.3: Forces due to suction, acting on different surfaces
Figure 2.4: The stress state variables for an unsaturated soil (Fredlund and Rahardjo,
1993)
Figure 2.5: Mohr-Coulomb failure envelope for a saturated soil
Figure 2.6: Planar modified Mohr-Coulomb failure envelope as a planar surface for
an unsaturated soil (after Fredlund and Rahardjo, 1993)
Figure 2.7: Shear strength versus matric suction (a) for a compacted glacial till with
a net normal stress 72.2kPa (modified after Gan et al. (1988)), (b) for different net-
normal stress 120kPa & 600kPa (modified after Escario and Juca (1989))21
Figure 2.8: Non-linear shear strength envelope for an unsaturated soil (after
Fredlund and Rahardjo, 1993)21
Figure 2.9: Non-planar modified Mohr-Coulomb failure envelope with respect to
matric suction for an unsaturated soil (after Fredlund and Rahardjo, 1993)22
Figure 2.10: Shear strength versus matric suction for decomposed granite (SM)
mixed with different percentages of kaolin at normal stress of 15.5kPa (after
Jotisankasa et al., (2010))
Figure 2.11: Typical soil water characteristic curves for desorption and adsorption
for unsaturated soils (after Fredlund and Rahardjo, 1993)
Figure 2.12: Conceptual soil water characteristic curves (SWCCs) of different soil
types (after Fredlund and Rahardjo, 1993)
Figure 2.13: Miniature KU tensiometer sensor
Figure 2.14: KU tensiometer and its incorporation in direct shear box (modified
from Jotisankasa and Mairaing, 2010)
Figure 2.15: Experiment setup for the continuous measurement of soil water
characteristic curve (SWCC) (after Jotisankasa et al., 2010)34
Figure 2.16: Drying SWCCs for a silty residual soil (after Jotisankasa et al., 2010) 34
Figure 2.17: Wetting SWCCs for a silty residual soil (after Jotisankasa et al., 2010)
35

Figure 2.18: Pressure plate (5-bar) extractor
Figure 2.19: Soil water characteristic curve (SWCC) for Pussellawa landslide soil
using pressure plate apparatus (after Tharanganie, 2004)
Figure 2.20: Soil water characteristic curve (SWCC) for Kahagalla landslide soil
using pressure plate apparatus (after Tharanganie, 2004)
Figure 2.21: The variation of apparent cohesion with matric suction for Pussellawa
landslide soil (after Tharanganie, 2004)
Figure 2.22: The variation of apparent cohesion with matric suction for Kahagalla
landslide soil (after Tharanganie, 2004)
Figure 2.23: Typical results during drying path for a silty residual soil (after
Jotisankasa et al., 2010)40
Figure 2.24: Typical results during wetting path for a silty residual soil (after
Jotisankasa et al., 2010)40
Figure 2.25: Typical results of permeability function for a silty residual soil (after
Jotisankasa et al., 2010)41
Figure 2.26: Particle size distribution of soil materials on which moisture
characteristic data were obtained. Dashed lines indicate extrapolation (after Arya &
Paris, 1981)43
Figure 2.27: Model parameter $\alpha$ as a function of particle size for five soil materials
(after Arya & Paris, 1981)46
Figure 2.28: Cut slope (1:1) geometry, selected sections and boundary conditions .48
Figure 2.29: Characteristics of residual soils used
Figure 2.30: Variation of pore water pressure for rainfall of 5mm/hr (after Sujeevan
and Kulathilaka, 2011)49
Figure 2.31: Variation of pore water pressure for rainfall of 20mm/hr (after Sujeevan
and Kulathilaka, 2011)49
Figure 2.32: Variation of pore water pressure for rainfall of 20mm/hr (after Sujeevan
and Kulathilaka, 2011)49
Figure 2.33: Pore water pressure distribution for 5mm/hr rainfall with vegetation
layer of permeability 10 <sup>-7</sup> m/s (Case 1) (after Kulathilaka and Kumara, 2011)50
Figure 3.1: The research study area

Figure 3.2: Simplified geological map of Sri Lanka showing major lithotectonic
units (After Cooray, 1994)52
Figure 3.3: Particle size distribution for SILTY SAND55
Figure 3.4: Particle size distribution for SANDY SILT56
Figure 3.5: Details of permeability test for SILTY SAND
Figure 3.6: Details of permeability test for SANDY SILT
Figure 3.7: The variation of void ratio with stress applied59
Figure 4.1: Typical arrangement of 5-bar pressure plate apparatus used for the
research
Figure 4.2: Cross sectional view of a ceramic pressure plate cell
Figure 4.3: Soil water characteristic curve (SWCC) for SILTY SAND65
Figure 4.4: Soil water characteristic curve (SWCC) for SANDY SILT66
Figure 4.5: Comparison of soil water characteristic curve (SWCC) for both soil
types67
Figure 5.1: The variation of shear stress with shear strain for fully saturated
condition (SILTY SAND)70
Figure 5.2: The variation of volume change with shear strain for fully saturated
condition (SILTY SAND)70
Figure 5.3: The variation of shear stress with normal stress for fully saturated
condition (SILTY SAND)71
Figure 5.4: The variation of shear stress with shear strain for approximately 40% (as
it is) saturated condition (SILTY SAND)73
Figure 5.5: The variation of volume change with shear strain for approximately 40%
(as it is) saturated condition (SILTY SAND)73
Figure 5.6: The variation of shear stress with normal stress for approximately 40%
(as it is) saturated condition (SILTY SAND)74
Figure 5.7: The variation of shear stress with shear strain for approximately 65%
saturated condition (SILTY SAND)
Figure 5.8: The variation of volume change with shear strain for approximately 65%
saturated condition (SILTY SAND)76

Figure 5.9: The variation of shear stress with normal stress for approximately 65%
saturated condition (SILTY SAND)77
Figure 5.10: The variation of shear stress with shear strain for approximately 83%
saturated condition (SILTY SAND)79
Figure 5.11: The variation of volume change with shear strain for approximately
83% saturated condition (SILTY SAND)
Figure 5.12: The variation of shear stress with normal stress for approximately 83%
saturated condition (SILTY SAND)80
Figure 5.13: The variation of apparent cohesion with degree of saturation (SILTY
SAND)81
Figure 5.14: The variation of apparent cohesion with volumetric water content
(SILTY SAND)82
Figure 5.15: The variation of apparent cohesion with matric suction (SILTY SAND)
83
Figure 5.16: Basic components of KU tensiometer
Figure 5.17: All components connected for the process of data transferring from
tensiometer85
Figure 5.18: Schematic diagram of data transferring from tensiometer85
Figure 5.19: Multi-meter for snapshot reading of tensiometer
Figure 5.20: Water saturation and de-airing process in to the vacuum chamber87
Figure 5.21: Screwing the acrylic tube with O-ring in the sensor body to achieve
tight seal87
Figure 5.22: Typical direct shear test at equilibrium stage (without load),
consolidation and shearing stages (with load) with tensiometer
Figure 5.23: The variation of shear stress with shear strain for fully saturated
condition (SANDY SILT)90
Figure 5.24: The variation of volume change with shear strain for fully saturated
condition (SANDY SILT)90
Figure 5.25: The variation of shear stress with normal stress for fully saturated
condition (SANDY SILT) 91

Figure 5.26: Direct shear specimens after the testing for fully saturated condition
(SANDY SILT)92
Figure 5.27: The variation of matric suction with time for approximately 50%
saturated condition (SANDY SILT)94
Figure 5.28: The variation of shear stress with shear strain for approximately 65%
saturated condition (SANDY SILT)95
Figure 5.29: The variation of volume change with shear strain for approximately
65% saturated condition (SANDY SILT)95
Figure 5.30: The variation of shear stress with normal stress for approximately 65%
saturated condition (SANDY SILT)96
Figure 5.31: The variation of matric suction with time at equilibrium stage for
approximately 65% saturated condition (SANDY SILT)97
Figure 5.32: The variation of matric suction with time at consolidation stage for
approximately 65% saturated condition (SANDY SILT)98
Figure 5.33: The variation of matric suction with time at shearing stage for
approximately 65% saturated condition (SANDY SILT)98
Figure 5.34: The variation of matric suction with shear strain at shearing stage for
approximately 65% saturated condition (SANDY SILT)99
Figure 5.35: Direct shear specimen after the testing for approximately 65% saturated
condition (SANDY SILT)99
Figure 5.36: The variation of shear stress with shear strain for approximately 72%
saturated condition (SANDY SILT)100
Figure 5.37: The variation of volume change with shear strain for approximately
72% saturated condition (SANDY SILT)101
Figure 5.38: The variation of shear stress with normal stress for approximately 72%
saturated condition (SANDY SILT)102
Figure 5.39: The variation of matric suction with time at equilibrium stage for
approximately 72% saturated condition (SANDY SILT)103
Figure 5.40: The variation of matric suction with time at consolidation stage for
approximately 72% saturated condition (SANDY SILT)103

Figure 5.41: The variation of matric suction with time at shearing stage for
approximately 72% saturated condition (SANDY SILT)
Figure 5.42: The variation of matric suction with shear strain at shearing stage for
approximately 72% saturated condition (SANDY SILT)
Figure 5.43: Direct shear specimen after the testing for approximately 72% saturated
condition (SANDY SILT)
Figure 5.44: The variation of shear stress with shear strain for approximately 81%
saturated condition (SANDY SILT)
Figure 5.45: The variation of volume change with shear strain for approximately
81% saturated condition (SANDY SILT)
Figure 5.46: The variation of shear stress with normal stress for approximately 81%
saturated condition (SANDY SILT)
Figure 5.47: The variation of matric suction with time at equilibrium stage for
approximately 81% saturated condition (SANDY SILT)109
Figure 5.48: The variation of matric suction with time at consolidation stage for
approximately 81% saturated condition (SANDY SILT)
Figure 5.49: The variation of matric suction with time at shearing stage for
approximately 81% saturated condition (SANDY SILT)110
Figure 5.50: The variation of matric suction with shear strain at shearing stage for
approximately 81% saturated condition (SANDY SILT)110
Figure 5.51: Direct shear specimen after the testing for approximately 81% saturated
condition (SANDY SILT)
Figure 5.52: The variation of shear stress with shear strain for approximately 92%
saturated condition (SANDY SILT)
Figure 5.53: The variation of volume change with shear strain for approximately
92% saturated condition (SANDY SILT)
Figure 5.54: The variation of shear stress with normal stress for approximately 92%
saturated condition (SANDY SILT)
Figure 5.55: The variation of matric suction with time at equilibrium stage for
approximately 92% saturated condition (SANDY SILT)

Figure 5.56: The variation of matric suction with time at consolidation stage for
approximately 92% saturated condition (SANDY SILT)115
Figure 5.57: The variation of matric suction with time at shearing stage for
approximately 92% saturated condition (SANDY SILT)115
Figure 5.58: The variation of matric suction with shear strain at shearing stage for
approximately 92% saturated condition (SANDY SILT)116
Figure 5.59: Direct shear specimen after the testing for approximately 92% saturated
condition (SANDY SILT)116
Figure 5.60: The variation of apparent cohesion with degree of saturation (SANDY
SILT)
Figure 5.61: The variation of apparent cohesion with volumetric water content
(SANDY SILT)118
Figure 5.62: The variation of apparent cohesion with matric suction (SANDY SILT)
119
Figure 5.63: The variation of maximum shear strength (at failure) with matric
suction for different loading conditions (SANDY SILT)121
Figure 5.64: The variation of apparent cohesion with matric suction (SANDY SILT)
Figure 6.1: Arrangement of KU tensiometers in permeability function for
drying/wetting path test
Figure 6.2: Typical arrangement for permeability function test for drying path128
Figure 6.3: The variation of soil mass with time for SILTY SAND (drying path).128
Figure 6.4: The variation of matric suction with time for SILTY SAND (drying
path)129
Figure 6.5: The variation of volumetric water content with matric suction (SWCC)
for SILTY SAND (drying path)
Figure 6.6: The variation of hydraulic gradient with time for SILTY SAND (drying
path)130
Figure 6.7: The variation of hydraulic conductivity with matric suction for SILTY
SAND (drying path)
Figure 6.8: The variation of soil mass with time for SANDY SILT (drying path).131

Figure 6.9: The variation of matric suction with time for SANDY SILT (drying
path)131
Figure 6.10: The variation of volumetric water content with matric suction (SWCC)
for SANDY SILT (drying path)
Figure 6.11: The variation of hydraulic gradient with time for SANDY SILT (drying
path)
Figure 6.12: The variation of hydraulic conductivity with matric suction for SANDY
SILT (drying path)
Figure 6.13: Typical arrangement for permeability function test for wetting path .133
Figure 6.14: The variation of soil mass with time for SILTY SAND (linear variation
due to constant rate of dripping of water-wetting path)
Figure 6.15: The variation of matric suction with time for SILTY SAND (wetting
path)
Figure 6.16: The variation of volumetric water content with matric suction (SWCC)
for SILTY SAND (wetting path)
Figure 6.17: The variation of hydraulic gradient with time for SILTY SAND
(wetting path)135
Figure 6.18: The variation of hydraulic conductivity with matric suction for SILTY
SAND (wetting path)
Figure 6.19: The variation of soil mass with time for SANDY SILT (wetting path)
Figure 6.20: The variation of matric suction with time for SANDY SILT (wetting
path)
Figure 6.21: The variation of volumetric water content with matric suction (SWCC)
for SANDY SILT (wetting path)
Figure 6.22: The variation of hydraulic gradient with time for SANDY SILT
(wetting path)
Figure 6.23: The variation of hydraulic conductivity with matric suction for SANDY
SILT (wetting path)
Figure 6.24: The variation of hydraulic conductivity with matric suction for SILTY
SAND (drying path)

Figure 6.25: The variation of hydraulic conductivity with matric suction for SANDY
SILT (drying path)
Figure 6.26: The variation of hydraulic conductivity with matric suction for SILTY
SAND (wetting path)140
Figure 6.27: The variation of hydraulic conductivity with matric suction for SANDY
SILT (wetting path)
Figure 6.28: The incident of water stagnated on the surface of the tested specimen
during wetting path141
Figure 7.1: The variation of volumetric water content with matric suction (SWCC)
for SILTY SAND143
Figure 7.2: The variation of volumetric water content with matric suction (SWCC)
for SANDY SILT144
Figure 7.3: The variation of volumetric water content with matric suction (SWCC)
for SANDY SILT145
Figure 7.4: The variation of volumetric water content with matric suction (SWCC)
during drying path for SILTY SAND
Figure 7.5: The variation of volumetric water content with matric suction (SWCC)
during drying path for SANDY SILT146
Figure 7.6: The variation of volumetric water content with matric suction (SWCC)
during wetting path for SILTY SAND147
Figure 7.7: The variation of volumetric water content with matric suction (SWCC)
during wetting path for SANDY SILT147
Figure 7.8: The variation of percentage passing with particle size for SILTY SAND
Figure 7.9: The variation of volumetric water content with matric suction for SILTY
SAND
Figure 7.10: The variation of model parameter with particle diameter for SILTY
SAND
Figure 7.11: The variation of percentage passing with particle size for SANDY SILT
151

Figure 7.12: The variation of volumetric water content with matric suction for
SANDY SILT152
Figure 7.13: The variation of model parameter with particle diameter for SANDY
SILT
Figure 7.14: The variation of volumetric water content with matric suction (SWCC)
for SILTY SAND for various methods
Figure 7.15: The variation of volumetric water content with matric suction (SWCC)
for SANDY SILT for various methods

## List of tables

Table 3.1: Summary index property test for soil type 01(SILTY SAND)55
Table 3.2: Summary index property test for soil type 02 (SANDY SILT)55
Table 3.3: Specimen details for the consolidation test
Table 3.4: Variation of coefficient of consolidation
Table 4.1: The condition of the specimens used for the series for SILTY SAND64
Table 4.2: Variation of matric suction with volumetric water content for SILTY
SAND
Table 4.3: The condition of the specimens used for the series for SANDY SILT65
Table 4.4: Variation of matric suction with volumetric water content for SANDY
SILT66
Table 5.1: The condition of tested specimen for fully saturated condition (SILTY
SAND)69
Table 5.2: Shear stress and strain values at failure with normal stress for fully
saturated condition (SILTY SAND)71
Table 5.3: Shear strength parameters for fully saturated condition (SILTY SAND) 72
Table 5.4: The condition of tested specimen for approximately 40% (as it is)
saturated condition (SILTY SAND)72
Table 5.5: Shear stress and strain values at failure with normal stress for
approximately 40% (as it is) saturated condition (SILTY SAND)74
Table 5.6: Shear strength parameters for approximately 40% (as it is) saturated
condition (SILTY SAND)75
Table 5.7: The condition of tested specimen for approximately 65% saturated
condition (SILTY SAND)75
Table 5.8: Shear stress and strain values at failure with normal stress for
approximately 65% saturated condition (SILTY SAND)77
Table 5.9: Shear strength parameters for approximately 65% saturated condition
(SILTY SAND)
Table 5.10: The condition of tested specimen for approximately 83% saturated
condition (SILTY SAND)78

Table 5.11: Shear stress and strain values at failure with normal stress for
approximately 83% saturated condition (SILTY SAND)80
Table 5.12: Shear strength parameters for approximately 83% saturated condition
(SILTY SAND)81
Table 5.13: The variation of apparent cohesion with degree of saturation (SILTY
SAND)81
Table 5.14: The variation of apparent cohesion with volumetric water content
(SILTY SAND)82
Table 5.15: The variation of volumetric water content with matric suction and
apparent cohesion (SILTY SAND)83
Table 5.16: The condition of tested specimen for fully saturated condition (SANDY
SILT)
Table 5.17: Shear stress and strain values at failure with normal stress for fully
saturated condition (SANDY SILT)91
Table 5.18: Shear strength parameters for fully saturated condition (SANDY SILT)
Table 5.16. Shear strength parameters for fully saturated condition (SAND1 Sill1)
92
Table 5.19: The condition of tested specimen for approximately 50% saturated
92
Table 5.19: The condition of tested specimen for approximately 50% saturated
Table 5.19: The condition of tested specimen for approximately 50% saturated condition (SANDY SILT)
Table 5.19: The condition of tested specimen for approximately 50% saturated condition (SANDY SILT)
Table 5.19: The condition of tested specimen for approximately 50% saturated condition (SANDY SILT)
Table 5.19: The condition of tested specimen for approximately 50% saturated condition (SANDY SILT)
Table 5.19: The condition of tested specimen for approximately 50% saturated condition (SANDY SILT)
Table 5.19: The condition of tested specimen for approximately 50% saturated condition (SANDY SILT)
Table 5.19: The condition of tested specimen for approximately 50% saturated condition (SANDY SILT)
Table 5.19: The condition of tested specimen for approximately 50% saturated condition (SANDY SILT)
Table 5.19: The condition of tested specimen for approximately 50% saturated condition (SANDY SILT)
Table 5.19: The condition of tested specimen for approximately 50% saturated condition (SANDY SILT) 93  Table 5.20: The condition of tested specimen for approximately 65% saturated condition (SANDY SILT) 94  Table 5.21: Shear stress and strain values at failure with normal stress for approximately 65% saturated condition (SANDY SILT) 96  Table 5.22: Shear strength parameters for approximately 65% saturated condition (SANDY SILT) 97  Table 5.23: The condition of tested specimen for approximately 72% saturated condition (SANDY SILT) 100  Table 5.24: Shear stress and strain values at failure with normal stress for

Table 5.26: The condition of tested specimen for approximately 81% saturated
condition (SANDY SILT)
Table 5.27: Shear stress and strain values at failure with normal stress for
approximately 81% saturated condition (SANDY SILT)107
Table 5.28: Shear strength parameters for approximately 81% saturated condition
(SANDY SILT)
Table 5.29: The condition of tested specimen for approximately 92% saturated
condition (SANDY SILT)111
Table 5.30: Shear stress and strain values at failure with normal stress for
approximately 92% saturated condition (SANDY SILT)113
Table 5.31: Shear strength parameters for approximately 92% saturated condition
(SANDY SILT)
Table 5.32: The variation of apparent cohesion with degree of saturation (SANDY
SILT)
Table 5.33: The variation of apparent cohesion with volumetric water content
(SANDY SILT)
Table 5.34: The variation of volumetric water content with matric suction and
apparent cohesion (SANDY SILT)
Table 5.35: The variation of shear strength, normalized shear strength with matric
suction for different saturated condition (SANDY SILT)120
Table 7.1: The variation of volumetric water content with matric suction for
different saturation level and normal stress for before and after the shearing for
SANDY SILT (from direct shear test data)
Table 7.2: The parameters used in the Arya & Paris method

## List of symbols

A - Cross sectional area of the sample

A\* - Point corresponds to the air-entry value

B\* - Point corresponds to residual Water Content

c<sub>a</sub> - Apparent cohesion

c<sup>s</sup> - Additional cohesion in unsaturated soil due to matric suction

c' - Effective cohesion

C<sub>v</sub> - Coefficient of consolidation

dt - Change of time

 $dV_{\rm w}$  - Change of volume of water

dz - Change of elevation head

e - Void ratio of the soil

e<sub>0</sub> - Initial void ratio

e<sub>f</sub> - Final void ratio

g - Acceleration due to gravity

G<sub>s</sub> - Specific gravity of the soil

 $h_i$  - Total pore length

h<sub>t</sub> - Total head

H<sub>1</sub> - Height of drainage path for section- 1

H<sub>2</sub> - Height of drainage path for section- 2

i - Hydraulic gradient

I<sub>r</sub> - Rainfall intensity

k - Permeability of the soil

m - Soil mass

m<sub>v</sub> - Coefficient of volume compressibility

n<sub>i</sub> - Number of spherical particles

p - Normal stress applied for consolidation test

q - Boundary flux

Q - Nodal flux

r<sub>i</sub> - Pore radius

R<sub>i</sub> - Mean particle radius

R<sub>s</sub> - Radius of curvature

s - Matric suction

S<sub>r</sub> - Degree of saturation of the soil

t - Elapsed time

t<sub>90</sub> - Time taken for 90% consolidation

t<sub>90,1</sub> - Time taken for 90% consolidation for Section-1

t<sub>90,2</sub> - Time taken for 90% consolidation for Section-2

T<sub>90</sub> - Time factor for 90% consolidation

T<sub>s</sub> - Surface tension

u<sub>a</sub> - Pore air pressure

u<sub>w</sub> - Pore water pressure

 $(u_a - u_w)_b$  - Matric suction at air-entry value

 $(u_a - u_w)_{calc}$  - Matric suction calculated

 $(u_a - u_w)_{meas}$  - Matric suction measured

 $(u_a - u_w)_r$  - Matric suction at residual water content

v - Flux or discharge velocity

V - Volume of soil

V<sub>b</sub> - Sample bulk volume per unit sample mass

 $V_{pi}$  - Total solid volume

V<sub>t</sub> - Voltage at any suction/pressure at the time

V<sub>vi</sub> - Pore volume per unit sample mass in i<sup>th</sup> particle size range

 $V_{\rm w}$  - Volume of water of soil

V<sub>0</sub> - Voltage at atmospheric pressure

w - Gravimetric moisture content of the soil

W<sub>i</sub> - Solid mass per unit sample mass in i<sup>th</sup> particle site range

z - Elevation head of each tensiometer relative to the base of sample

W<sub>s</sub> - Solid weight of the soil

 $\gamma_{\rm w}$  - Unit weight of the water

θ<sub>r</sub> - Residual volumetric water content

 $\theta_s$  - Saturated volumetric water content

 $\theta_{\rm w}$  - Volumetric water content

 $\theta$  - Contact angle

 $(\sigma_n - u_a)$  - Net normal stress

 $\tau$  - Shear stress

φ' - Effective internal angle of friction

 $\sigma_n$  - Normal stress

 $\sigma'$ ,  $(\sigma_n - u_w)$  - Effective stress

 $\tau_{max}$  - Shear strength at failure

φ<sup>b</sup> - Angle of shearing resistance due to suction

 $\sigma_x$  - Total normal stress in the x-direction (or on the x-plane)

 $\sigma_y$  - Total normal stress in the y-direction (or on the y-plane)

 $\sigma_z$  - Total normal stress in the z-direction (or on the z-plane)

 $(\sigma_x - u_a)$  - Net normal stress in the x-direction

 $(\sigma_{y}$  -  $u_{a})$   $\;\;$  - Net normal stress in the y-direction

 $(\sigma_z - u_a)$  - Net normal stress in the z-direction

 $\tau_{xy}\,$  - Shear stress on the x-plane in the y-direction

 $\tau_{xz}$  - Shear stress on the x-plane in the z-direction

 $\tau_{yx}$  - Shear stress on the y-plane in the x-direction

 $\tau_{zx}$  - Shear stress on the z-plane in the x-direction

 $\tau_{zy}$  - Shear stress on the z-plane in the y-direction

 $\rho_p$  - Particle density

 $\theta_{vi}$  - Volumetric water content  $i^{th}$  particle size range

 $\theta^*_{vi}$  - Average volumetric water content represent by mid-point of the  $i^{th}$  particle size range

 $\theta_{vi+1}$  - Volumetric water content  $(i+1)^{th}$  particle size range

 $\pi$  - Mathematical constant (Pi)

 $\alpha$  - Model parameter

 $\rho_{\rm w}$  - Density of water

 $\rho_d$  - Dry density of soil

 $\rho_{\rm w}$  - Density of water

 $\psi$  - Soil water pressure head

### List of abbreviations

Al - Aluminum

Al<sub>2</sub>O<sub>3</sub> - Aluminum Oxide

ATM - Atmospheric pressure

ATT - At The Time

BSCS - British Soil Classification System

Cr - Chromium

E01 - Express way No.1

Fe - Iron

Fe<sub>2</sub> O<sub>3</sub> - Ferric Oxide

GED - Geotechnical Engineering Division

GIL - Geotechnical Innovation Laboratory

HC - Highland Complex

KC - Kadugannawa Complex

KU - Kasetsart University

KU T1 - Kasetsart University Tensionmeter type 1

KU T2 - Kasetsart University Tensionmeter type 2

KU T3 - Kasetsart University Tensionmeter type 3

KU T4 - Kasetsart University Tensionmeter type 4

LCD - Liquid Crystal Display

MEMs - Micro Electro Mechanical System

Mn - Manganese

MH - SANDY elastic SILT

MS - SANDY SILT

NBRO - National Building Research Organization

Ni - Nickel

PVC - Poly Vinyl Chloride

SD - Secure Digital

SM - SILTY SAND

SWCC - Soil Water Characteristic Curve

VC - Vijayan Complex

WC - Wanni Complex

1-D - One Dimensional

## **Annexures**

Annex 1: Particle size distribution test results	164-165
Annex 2: Direct shear test results.	166-175
Annex 3: Matric suction results during direct shear tests	176-177
Annex 4: Permeability function results	178-179
Annex 5: Arva & Paris method results	180-182